

- * The coefficients in the E one-side line will yield the maximum moments that can occur if the column is loaded on only one side, such as the end column in a bay.

5.9.3 Slenderness Effects in Columns and Wall Panels

Sections 10.10 through 10.13 of ACI 318-05 contain provisions for evaluating slenderness effects (buckling) of columns. Additional recommendations are given in the recommended practice section of this handbook (Section 14.1).

Slenderness effects can be described as the moments in a component produced when the line of action of the axial force is not coincident with the displaced centroid of the component. These moments, which are not accounted for in the primary analysis, are thus termed *secondary moments*. These secondary moments arise from changes in the geometry of the structure, and may be caused by one or more of the following:

1. Relative displacement of the ends of the component due to:
 - a. Lateral or unbalanced vertical loads in an unbraced frame, usually labeled as translation or sidesway
 - b. Manufacturing and erection tolerances
2. Deflections away from the end of the component due to:
 - a. End moment due to eccentricity of the axial load
 - b. End moments due to frame action-continuity, fixity, or partial fixity of the ends
 - c. Applied lateral loads, such as wind
 - d. Thermal bowing from differential temperature (see Section 5.8.5)
 - e. Manufacturing tolerances
 - f. Camber due to prestressing

Section 10.10.1 of ACI 318-05 describes the use of second-order analysis for slenderness effects. ACI 318-05 Section 10.10.2 also allows the use of an approximate procedure called moment magnification. Precast concrete frames with columns reinforced with non-prestressed reinforcement that meet the requirements for minimum reinforcement may be designed using moment magnification, but prestressed compression components usually have less than the minimum 1% vertical reinforcement. The recommended practice suggests ways to modify the code equations used in moment magnification, but the second-order, or $P-\Delta$, analysis is preferred.

5.9.3.1 Second-Order ($P-\Delta$) Analysis

The usual procedure for this analysis is to perform an elastic-type analysis using factored loads. While deflections are usually only a concern under service load, the deflections calculated for this purpose are to avoid a stability failure, so it is logical to provide the same safety factor as for strength design.

Out-of-plumbness (items 1b and 2e noted in Section 5.9.3) are initially assumed based on experience and/or specified tolerances. The thermal bowing effect is usually neglected in

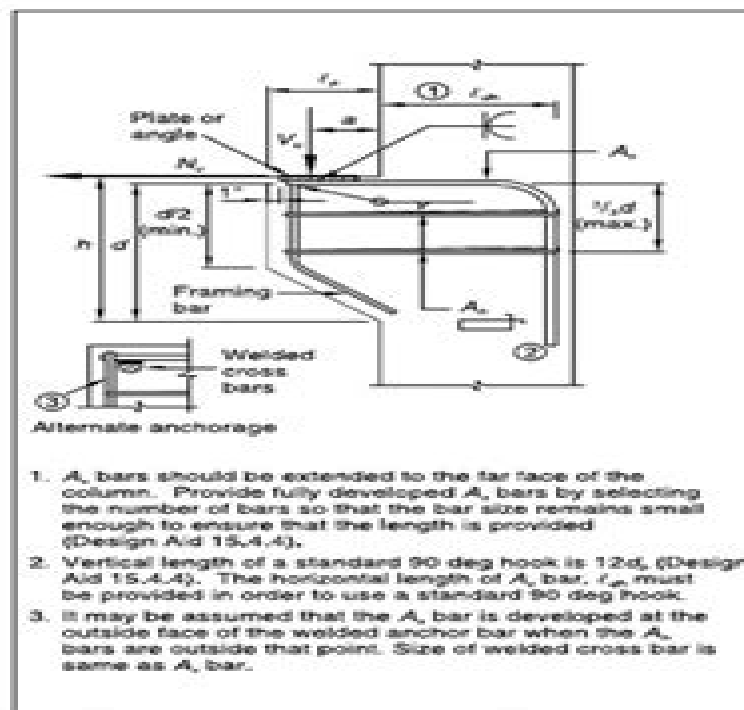


Fig. 5.9.2 Design of concrete corbels. Note: max. = maximum; min. = minimum.

columns, but may be significant in exterior wall panels (see Section 5.8.5).

At each iteration, the lateral deflection is calculated, and the moments caused by the axial load acting at that deflection are accumulated. After three or four iterations, the increase in deflection should be negligible (convergence). If it is not, the component may be approaching stability failure, and the section dimensions should be reevaluated.

If the calculated moments indicate that cracking will occur, this needs to be taken into account in the deflection calculations. The stiffness used in the second-order analysis should represent the stiffness of the component immediately before failure. This may involve iterations within iterations, greatly complicating the procedure. Assuming the panel remains uncracked, EI needs to be modified by a stiffness reduction factor ϕ , and effects of creep should also be included. The most common method is to divide the stiffness EI by the factor $1 + \beta_s$ as specified in the ACI 318-05 moment-magnifi-

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